

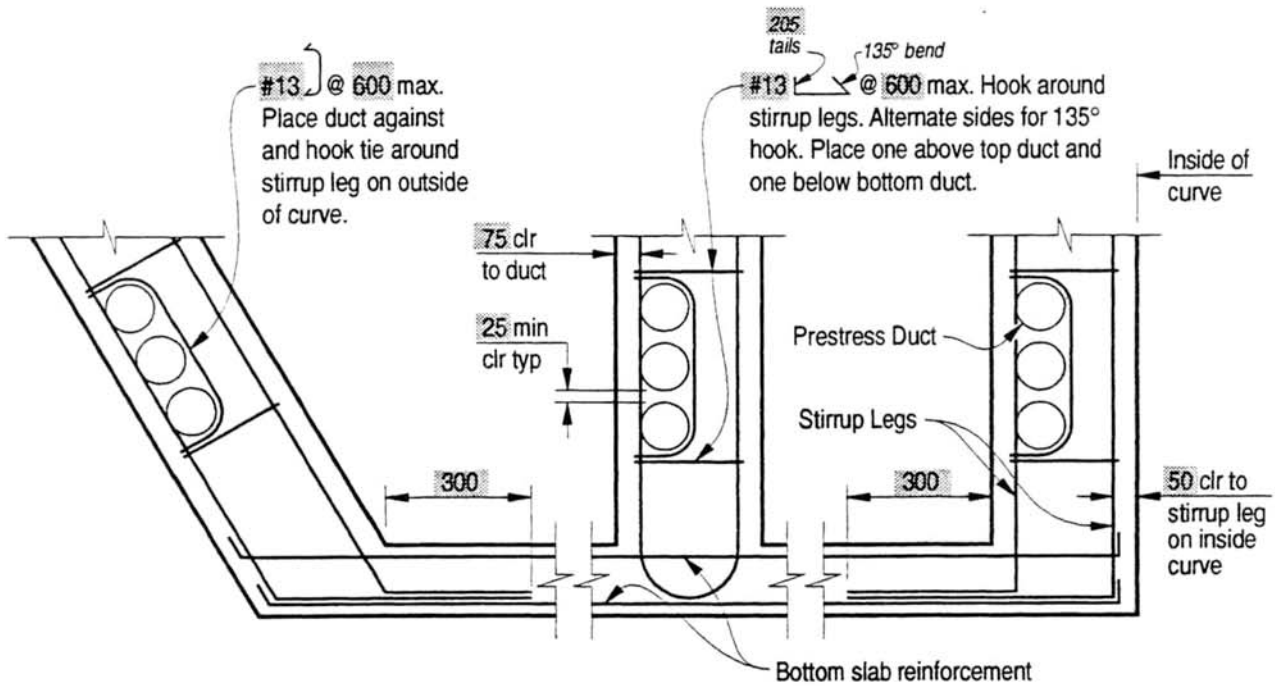
## 11-31 CURVED POST-TENSIONED BRIDGES

Designers should consider the lateral prestress force on any curved post-tensioned structure. Calculate this force ( $F$ ) for each girder, by dividing the jacking force ( $P_j$ ) per girder by the horizontal radius ( $R$ ) of the girder.

All girders with a  $P_j/R > 100$  kN per m per girder or a horizontal radius ( $R$ ) of 250 m or less shall use Detail "A" shown on page 2 of this memo.

The included graphs may be used to check girder webs for containment of tendons and adequate stirrup reinforcement to resist flexural bending.

The last section of this memo provides a commentary on design assumptions.



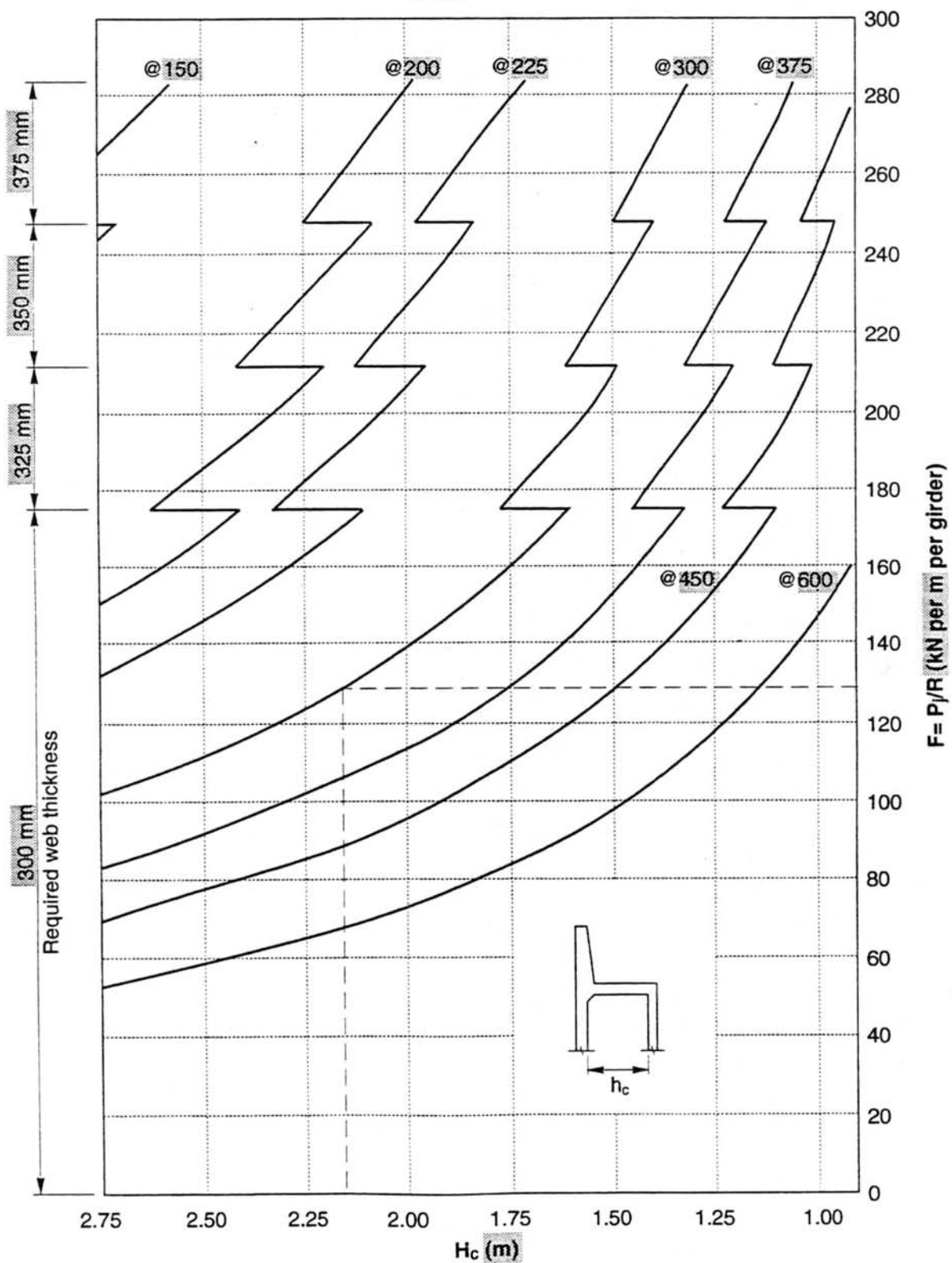
*Note:* Details shown are for a curve to the right with the section taken looking ahead on station. These details supersede duct patterns shown in Standard Plan B8-5.

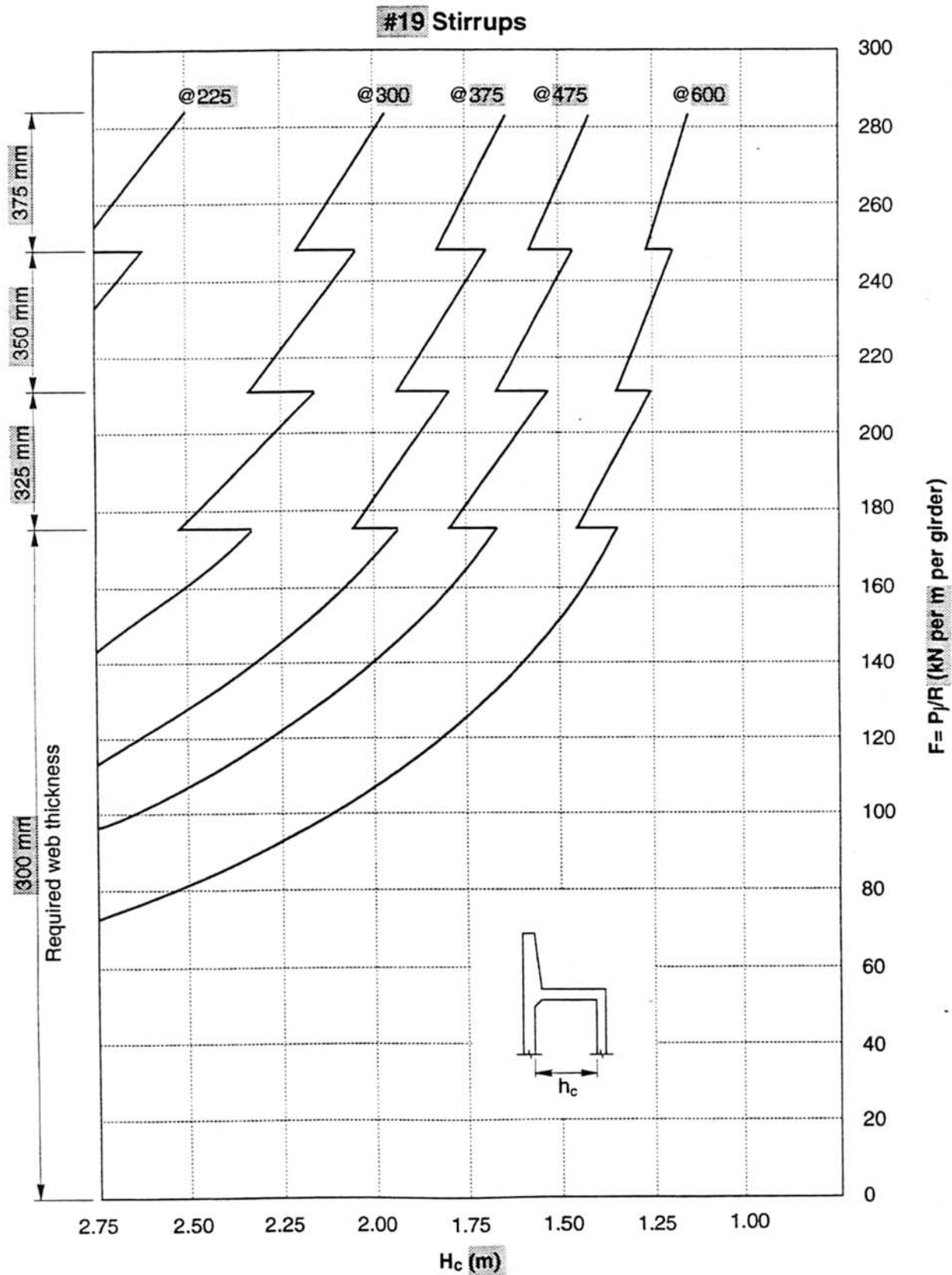
## PART GIRDER SECTION

### Detail "A"

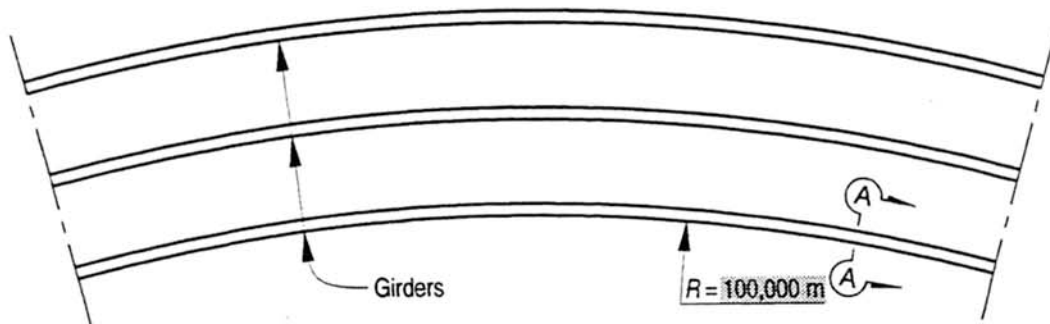
Use of Detail "A" will require an adjustment of the basic prestressing steel path ordinates used for design. See Memo to Designers 11-28.

#16 Stirrups

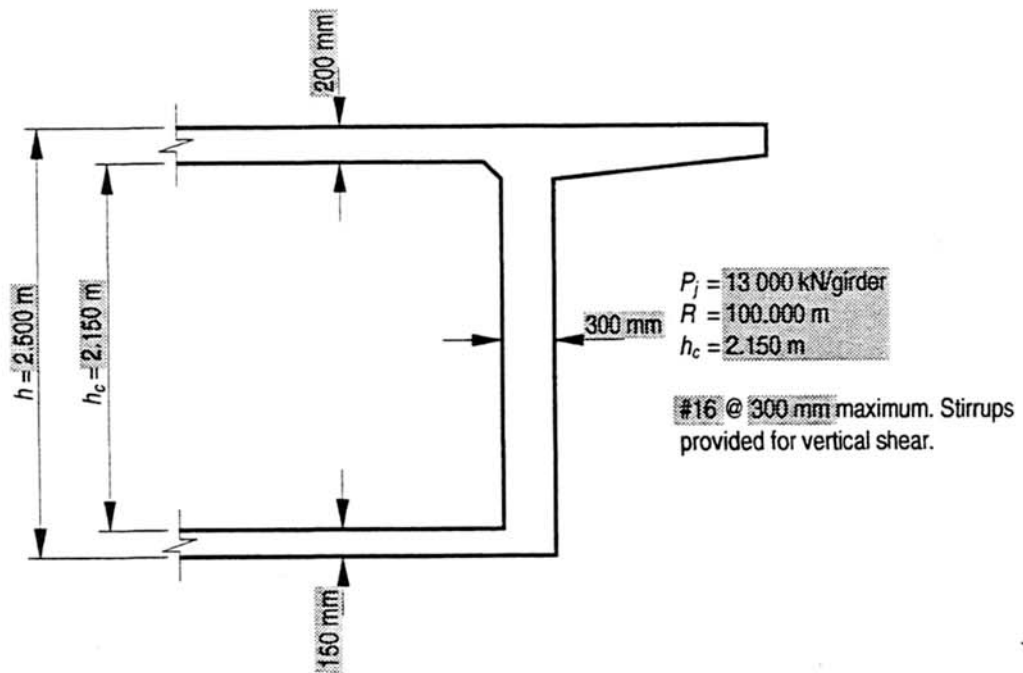




Example



PLAN



SECTION A-A

$$F = P_f / R = 13,000 / 100 = 130 \text{ kN per m}$$

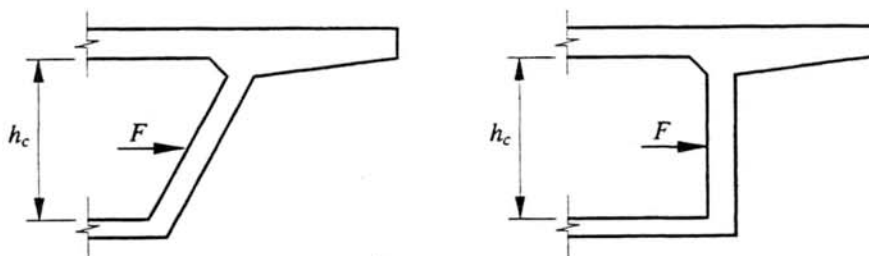
From the graph for #16 Stirrups,  $F = 130$  and  $h_c = 2,150$  — decrease maximum stirrup spacing from 300 mm to 220 mm.

## Commentary

The following assumptions were made in the development of this Memo to Designers:

### Bending Moment

The girder web is assumed to be a beam with a length equal to the clear distance between top and bottom slabs ( $h_c$ ). The lateral prestress force ( $F$ ) is acting at the center point of the web ( $h_c/2$ ). The moment is calculated by simple beam formula reduced 20% for continuity between web and slabs with 1.0 load factor applied.



$$M_u = (1.0)(0.8)(1/4)(P_j/R)(h_c) = 0.20 (F) h_c$$

The shear and bending stresses in the girder stirrups are additive. For the purposes of this memo, however, stirrups are considered capable of handling these stresses independently for the following reasons:

- $M_u$  is calculated for the maximum condition of  $F$  acting at  $h_c/2$ . This occurs at only two points in a span due to tendon drape.
- The jacking force,  $P_j$ , is used in the calculation of  $M_u$ , and at the time  $P_j$  is applied, the structures is supported on falsework. When the falsework is removed and vertical shear forces act, the prestressing force has been reduced by losses.

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